# Three Dimensional Elastic Frames: Rigid Joint Conditions In Variational And Differential Formulation

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Joint work with Gregory Berkolaiko

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### Overview

### **1** Introduction and Motivation

- Beam Structures in Practice
- Planar Frames and Matching Vertex Conditions
- Planar Frames and Matching Vertex Conditions
- 2 General Three Dimensional Graphs
  - Parameterization of Beam Deformation
  - Full Description of Euler-Bernoulli Energy Functional

### 8 Energy Form and Differential Operator

- Quadratic Form and Vertex Conditions
- Hamiltonian on Graph and Vertex Conditions
- Decoupling of Fields for Planar Graph
- Symmetry and Irreduscible Representations
  - Numerical Results and Discussion
  - Numerical Results and Discussion

# Section 1

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  - Parameterization of Beam Deformation
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- 4 Symmetry and Irreduscible Representations
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### **Beam Structures in Practice**

Mathematical modeling of vibration of structures made of joined together beams is a topic of natural interest for engineers (pic from net).

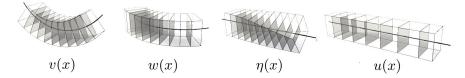


### **Beam Structures in Practice**

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② Each beam is described by (Euler-Bernoulli) energy functional considering energy of:



# Matching Vertex Conditions in Planar Graph

• Energy functional corresponding to network of beams  $\Gamma = (E, V)$  is given by

$$\Pi = \frac{1}{2} \sum_{e \in E} \int_{e} a_{e}(x) |v_{e}''(x)|^{2} dx.$$

- Domain of  $\Pi$  consists of functions  $v\in \bigoplus_{e\in E} H^2(e)$  that satisfy certain vertex conditions.
- One way is to assign analogue of *standard* vertex conditions introduced for the Laplacian (e.g. see the work by B. Dekoninck and S. Nicaise, 2000)

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- In order to study the spectral gap, the following vertex conditions are assumed (see the work by P. Kurasov and J. Muller, 2020):

$$v_i(c) = v_j(c)$$
 when  $e_i, e_j$  adjacent to  $c$   
 $v_i^\prime(c) = 0$ 

2 The corresponding Beam operators, mapping  $v_e \mapsto a_e v_e'''$ , is defined from  $v \in \bigoplus_{e \in E} H^4(e)$  satisfying at each vertex

$$v_i(c) = v_j(c)$$
 when  $e_i, e_j$  adjacent to  $c$   
 $v'_i(c) = 0$   
 $\sum_{e_i \sim c} v''_i(c) = 0$ 

Introduction and Motivation General Three Dimensional Beam Structures in Practice Planar Frames and Matching

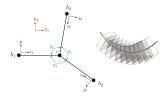
### Matching Vertex Conditions in Planar Graph

Energy functional and corresponding vertex conditions

$$\Pi = \frac{1}{2} \sum_{e \in E} \int_{e} a_{e}(x) |v_{e}''(x)|^{2} dx.$$

Vertex conditions on the quadratic form (see the work by J.C. Kiik, P. Kurasov, and M. Usman, 2015)

- $v_1(c) = v_2(c) = v_3(c)$
- $\sin(\theta_1)v_1'(c) + \sin(\theta_2)v_2'(c) + \sin(\theta_3)v_3'(c) = 0$



Introduction and Motivation General Three Dimensional Beam Structures in Practice Planar Frames and Matching

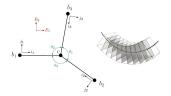
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**2** Corresponding self-adjoint Beam operator, mapping  $v_e \mapsto a_e v_e'''$ , on every edge  $e \in E$  satisfying (in addition to above conditions)

• 
$$\frac{v_1''(c)}{\sin(\theta_1)} = \frac{v_2''(c)}{\sin(\theta_2)} = \frac{v_3''(c)}{\sin(\theta_3)}$$

• 
$$v_1^{\prime\prime\prime}(c) + v_2^{\prime\prime\prime}(c) + v_3^{\prime\prime\prime}(c) = 0$$

### O Role of Degrees of Freedom

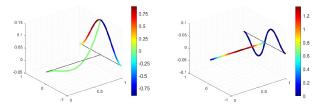


Figure: Eigenfunctions corresponding first and second eigenvalues.

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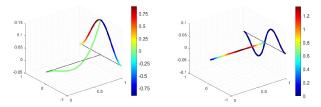
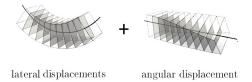
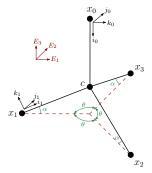


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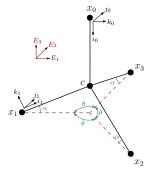
#### From Scalar to Vector Quantities



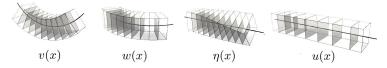
• Generalization to three dimensional structures



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Including all Degrees of Freedom



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### 2 General Three Dimensional Graphs

- Parameterization of Beam Deformation
- Full Description of Euler-Bernoulli Energy Functional

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## Parameterization of Beam Deformation

- Euler-Bernoulli hypothesis:
  - Plane sections remain plane,
  - Geometry of the spatial beam is described by the **centroid line** and a family of the corresponding **cross-sections**.



# Parameterization of Beam Deformation

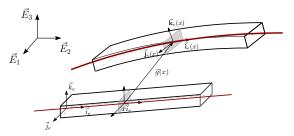
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- Ø Description of problem in basis:
  - Orthonormal basis  $\{\vec{E}_1,\vec{E}_2,\vec{E}_3\}$  span the physical space in which the beam is embedded,
  - Orthonormal basis  $\{ec{i},ec{j},ec{k}\}$  describes orientation of the cross section of beam
- Of Deformed configuration fully described by:
  - Position vector  $\vec{g}(x)$  with x representing the arc-length coordinate,
  - Family of orthonormal basis  $\{\vec{i}(x), \vec{j}(x), \vec{k}(x)\}$  which describe the orientation of the cross sections in the **deformed** configuration.

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# **Rigid Vertex Condition**

**9** The relationship between the cross-section basis in the initial un-deformed and the deformed configurations can be expressed through  $\mathcal{R}(x) \in SO(3)$ 

$$\vec{i}(x) = \mathcal{R}(x)\vec{i}, \qquad \vec{j}(x) = \mathcal{R}(x)\vec{j}, \qquad \vec{k}(x) = \mathcal{R}(x)\vec{k}$$

2 Introduce (linearized)-rotation vector  $\vec{\omega}(x) := \alpha \vec{\vartheta}(x)$ 

$$\vec{i}(x) = \vec{i} + \vec{\omega}(x) \times \vec{i}, \qquad \vec{j}(x) = \vec{j} + \vec{\omega}(x) \times \vec{j}, \qquad \vec{k}(x) = \vec{k} + \vec{\omega}(x) \times \vec{k}$$

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### Definition

A joint v with n incident beans  $\{e_i\}_{i=1}^n$  is called **rigid**, if the displacement and rotation vectors on beams  $e_i$  satisfy

$$\vec{g}_1(v) = \cdots = \vec{g}_n(v), \quad \text{and} \quad \vec{\omega}_1(v) = \cdots = \vec{\omega}_n(v)$$

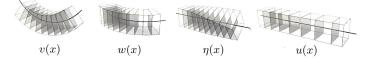


# Towards General Graph $\Gamma = (V, E)$

• Kinematic Bernoulli assumptions for beam frame:

• no vertex energy, pre-sress, or external force

$$\mathcal{U}^{(\Gamma)} = \frac{1}{2} \sum_{e \in E} \int_{e} \left( a_{e}(x) |v_{e}''(x)|^{2} + b_{e}(x) |w_{e}''(x)|^{2} + c_{e}(x) |u_{e}'(x)|^{2} + d_{e}(x) |\eta_{e}'(x)|^{2} \right) dx.$$



② Associated to each edge  $e \in E$  is a local orthonormal basis  $\{\vec{i}_e, \vec{j}_e, \vec{k}_e\}$ 

•  $\vec{g}_e(x)$  is displacement vector of edge e in global coordinate system at  $x \in e$ .

$$(u_e, w_e, v_e)(x) := \left(\vec{g}_e \cdot \vec{i}_e, \vec{g}_e \cdot \vec{j}_e, \vec{g}_e \cdot \vec{k}_e\right)(x)$$

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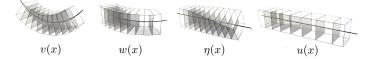
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A key ingredient of generalization to elastic frame is the property of vertex at which the incident edges are met.

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### **Quadratic Form and Vertex Conditions**

#### Theorem

Energy functional  $\Pi$  of the beam frame with free rigid joints is the quadratic form corresponding to the positive closed **sesquilinear form** 

$$h\left[\widetilde{\Psi},\Psi\right] := \sum_{e \in E} \int_{e} \left(a_e \overline{\widetilde{v}''_e} v''_e + b_e \overline{\widetilde{w}''_e} w''_e + c_e \overline{\widetilde{u}'_e} u'_e + d_e \overline{\widetilde{\eta}'_e} \eta'_e\right) dx_e$$

densely defined on

$$\mathcal{H} = \bigoplus_{e \in E} L_2(e) \times \bigoplus_{e \in E} L_2(e) \times \bigoplus_{e \in E} L_2(e) \times \bigoplus_{e \in E} L_2(e)$$

with the **domain** of h consisting of the vectors

$$\Psi := \begin{pmatrix} v, & w, & u, & \eta \end{pmatrix}^T \in \bigoplus_{e \in E} H^2(e) \times \bigoplus_{e \in E} H^2(e) \times \bigoplus_{e \in E} H^1(e) \times$$

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that satisfy, at every vertex  $v \in V$ , the "free rigid joint" conditions

• continuity of displacement,

$$u_1\vec{i}_1 + w_1\vec{j}_1 + v_1\vec{k}_1 = \dots = u_n\vec{i}_n + w_n\vec{j}_n + v_n\vec{k}_n$$

• continuity of rotation,

$$\eta_1 \vec{i}_1 - v'_1 \vec{j}_1 + w'_1 \vec{k}_1 = \dots = \eta_n \vec{i}_n - v'_n \vec{j}_n + w'_n \vec{k}_n$$

where n is the degree of v.

## Hamiltonian on Graph and Vertex Conditions

#### Theorem

Energy form  $\Pi$  on a beam frame with free rigid joints corresponds to the **self-adjoint** operator  $H: \mathcal{H} \to \mathcal{H}$  acting as

$$\Psi_e := \begin{pmatrix} v_e \\ w_e \\ u_e \\ \eta_e \end{pmatrix} \mapsto \begin{pmatrix} a_e v_e^{\prime\prime\prime\prime} \\ b_e w_e^{\prime\prime\prime\prime} \\ -c_e u_e^{\prime\prime} \\ -d_e \eta_e^{\prime\prime} \end{pmatrix}$$

on every edge  $e \in E$  of the graph. The **domain** of the operator H consists of the functions

$$\Psi \in \bigoplus_{e \in E} H^4(e) \times \bigoplus_{e \in E} H^4(e) \times \bigoplus_{e \in E} H^2(e) \times \bigoplus_{e \in E} H^2(e)$$

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that satisfy, at each vertex  $v \in V$ ,

• continuity of displacement and rotation conditions respectively

$$u_1\vec{i}_1 + w_1\vec{j}_1 + v_1\vec{k}_1 = \dots = u_{n_v}\vec{i}_{n_v} + w_{n_v}\vec{j}_{n_v} + v_{n_v}\vec{k}_{n_v},$$
  
$$\eta_1\vec{i}_1 - v'_1\vec{j}_1 + w'_1\vec{k}_1 = \dots = \eta_{n_v}\vec{i}_{n_v} - v'_{n_v}\vec{j}_{n_v} + w'_{n_v}\vec{k}_{n_v},$$

• equilibrium of forces and moments, respectively

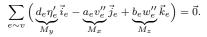
$$\sum_{e \sim v} \left( c_e u'_e \vec{i}_e - b_e w'''_e \vec{j}_e - a_e v'''_e \vec{k}_e \right) = \vec{0},$$
$$\sum_{e \sim v} \left( d_e \eta'_e \vec{i}_e - a_e v''_e \vec{j}_e + b_e w''_e \vec{k}_e \right) = \vec{0}.$$

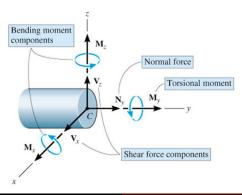
### Self-adjointness and Physics Behind Vertex Conditions

Equilibrium of forces at vertex

$$\sum_{e \sim v} \left( \underbrace{c_e u'_e}_{N_y} \vec{i}_e - \underbrace{b_e w'''_e}_{V_x} \vec{j}_e - \underbrace{a_e v''_e}_{V_z} \vec{k}_e \right) = \vec{0}$$

2 Equilibrium of moments at vertex





### **Decoupling of Fields for Planar Graph**

### Corollary

Free planar network of beams is described by Hamiltonian

$$\mathcal{H}^{(\Gamma)}(v,\eta,w,u) = \left(\mathcal{H}^{(\Gamma)}_1(v,\eta)\right) \oplus \left(\mathcal{H}^{(\Gamma)}_2(w,u)\right)$$

where  $\mathcal{H}_1^{(\Gamma)}$  and  $\mathcal{H}_2^{(\Gamma)}$  are differential operators with action

$$\mathcal{H}_1^{(\Gamma)}(v,\eta)\big|_e = \begin{pmatrix} a_e \frac{d^4}{dx^4} & 0\\ 0 & -d_e \frac{d^2}{dx^2} \end{pmatrix}, \quad \text{ and } \quad \mathcal{H}_2^{(\Gamma)}(w,u)\big|_e = \begin{pmatrix} b_e \frac{d^4}{dx^4} & 0\\ 0 & -c_e \frac{d^2}{dx^2} \end{pmatrix}$$

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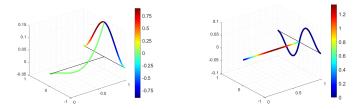
$$\begin{aligned} \mathcal{H}_1^{(1')} \text{ satisfying at each vertex } v \\ v_1 &= \cdots = v_{n_v}, \quad \text{and} \quad \eta_1 \vec{i}_1 - v_1' \vec{j}_1 = \cdots = \eta_{n_v} \vec{i}_{n_v} - v_{n_v}' \vec{j}_{n_v}, \\ &\sum_{e \sim v} d_e \eta_e' \vec{i}_e - a_e v_e'' \vec{j}_e = \vec{0}, \quad \text{and} \quad \sum_{e \sim v} a_e v_e''' = 0. \end{aligned}$$

•  $\mathcal{H}_2^{(\Gamma)}$  satisfying at each vertex v

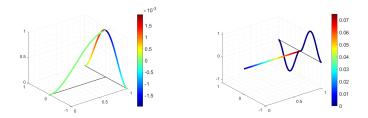
$$\begin{split} w_1' &= \cdots = w_{n_v}', \quad \text{and} \quad u_1 \vec{i}_1 + w_1 \vec{j}_1 = \cdots = u_{n_v} \vec{i}_{n_v} + w_{n_v} \vec{j}_{n_v}, \\ &\sum_{e \sim v} c_e u_e' \vec{i}_e - b_e w_e''' \vec{j}_e = \vec{0}, \quad \text{and} \quad \sum_{e \sim v} b_e w_e'' = \vec{0}. \end{split}$$

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## Example: Planar Graph



**Figure:** Eigenfuctions corresponding to first and second eigenvalues for parameters  $a = d = d_0 = 1$ . Color bar shows value of in-axis torsion of edges.



**Figure:** Eigenfuctions corresponding to first and second eigenvalues for parameters b = 1 and  $d = d_0 = 10^3$ . Color bar shows value of in-axis torsion of edges.

## Section 4

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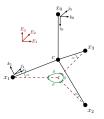
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# Irreduscible Representations

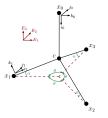


- **9** The graph  $\Gamma_{AT}$  is **invariant** under the symmetry group  $G = D_3$ , the dihedral group of degree 3.
  - R : rotation of  $\Gamma$  by  $\theta = 2\pi/3$  with axis of rotation along  $E_3$
  - F : reflection with respect to the planes passing through vertices  $x_1, x_0$  and c.
- ${f 0}$  The group G then can be described as

$$G = \langle R, F \mid R^3 = I, F^2 = I, FRFR = I \rangle$$

with I to be the identity element. This implies that G contains the elements

$$G = \{I, R, R^2, F, FR, FR^2\}$$



#### Theorem

The Hamiltonian operator H of the beam frame  $\Gamma_{AT}$  is reduced by the decomposition  $\mathcal{H} = \mathcal{H}_{id} \oplus \mathcal{H}_{alt} \oplus \mathcal{H}_{\omega} \oplus \mathcal{H}_{\overline{\omega}},$ 

where

$$\begin{split} \mathcal{H}_{\mathrm{id}} &:= \left\{ \Psi \in \mathcal{H} \colon v_0 = w_0 = \eta_0 = 0, \; w_s = \eta_s = 0, \; v_1 = v_2 = v_3, \; u_1 = u_2 = u_3 \right\}, \\ \mathcal{H}_{\mathrm{alt}} &:= \left\{ \Psi \in \mathcal{H} \colon v_0 = w_0 = u_0 = 0, \; u_s = v_s = 0, \; w_1 = w_2 = w_3, \; \eta_1 = \eta_2 = \eta_3 \right\}, \\ \mathcal{H}_{\omega} &:= \left\{ \Psi \in \mathcal{H} \colon u_0 = \eta_0 = 0, \; w_0 = \mathrm{i}v_0, \; \Psi_3 = \omega \Psi_2 = \omega^2 \Psi_1 \right\} = \overline{\mathcal{H}_{\omega}}, \\ \text{where } s \in \{1, 2, 3\} \; \text{labels the legs,} \; \Psi_s := (v_s, w_s, u_s, \eta_s)^T, \; \text{and} \; \omega = e^{2\pi\mathrm{i}/3}. \end{split}$$

### Remark

- A decomposition  $\mathcal{H} = \bigoplus_{\alpha} \mathcal{H}_{\alpha}$  is reducing for an operator H if
  - $\bullet \ H$  is  ${\rm invariant}$  on each of the subspaces, and
  - ${ullet}$  the operator domain  ${\rm Dom}(H)$  is aligned with respect to the decomposition, namely

$$\operatorname{Dom}(H) = \bigoplus_{\alpha} (\mathcal{H}_{\alpha} \cap \operatorname{Dom}(H)).$$

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- This means that we can **restrict** H to each subspace in turn and every aspect of the spectral data of the operator H is the **sum** (or union) of the spectral data of the restricted parts.
- In particular, since  $\mathcal{H}_{\omega} = \overline{\mathcal{H}_{\omega}}$ , the **eigenvalues** of the corresponding restrictions are **equal** and thus each eigenvalue of the restriction  $H_{\omega} = H|_{\mathcal{H}_{\omega}}$  enters the spectrum of H with **multiplicity two**.
- Kinematically, these eigenvalues correspond to the **rotational wobbles** of the antenna beam.

#### Theorem

The Hamiltonian operator H of the beam frame  $\Gamma_{AT}$  is reduced by the decomposition

 $\mathcal{H}=\mathcal{H}_{\mathrm{id}}\oplus\mathcal{H}_{\mathrm{alt}}\oplus\mathcal{H}_{\omega}\oplus\mathcal{H}_{\overline{\omega}},$ 

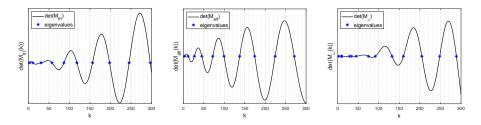
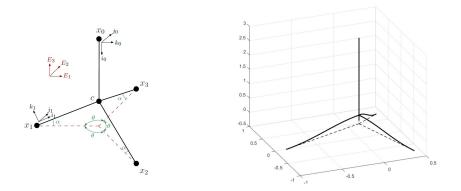


Figure: Variation of determinant of matrices corresponding irreducible representations and their corresponding eigenvalues: (left) trivial  $M_{\rm tri}$ , (middle) alternative  $M_{\rm alt}$ , and (right) standard  $M_{\omega}$ . All the results are based on unit materials parameters and beams lengths.

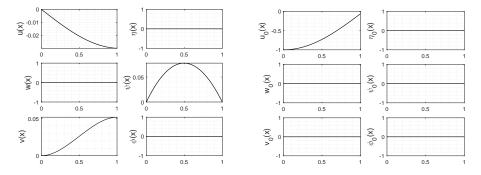
### $\mathcal{H}=\mathcal{H}_{\mathrm{id}}\oplus\mathcal{H}_{\mathrm{alt}}\oplus\mathcal{H}_{\omega}\oplus\mathcal{H}_{\overline{\omega}}$

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## $\mathcal{H} = \mathcal{H}_{\mathrm{id}} \oplus \mathcal{H}_{\mathrm{alt}} \oplus \mathcal{H}_{\omega} \oplus \mathcal{H}_{\overline{\omega}}$

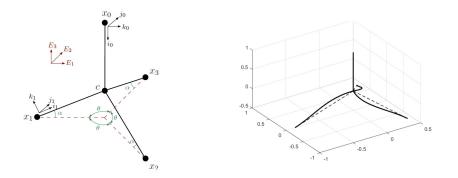
$$\mathcal{H}_{\mathrm{id}} := \left\{ \Psi \in \mathcal{H} \colon v_0 = w_0 = \eta_0 = 0, \ w_s = \eta_s = 0, \ v_1 = v_2 = v_3, \ u_1 = u_2 = u_3 \right\},$$



**Figure:** Plot of the components of the first eigenfuction from  $\mathcal{H}_{id}$ . Plots are obtained from a finite elements numerical computation and are displayed in the local coordinate system of the corresponding edge. All the results are based on unit materials parameters and beams lengths.

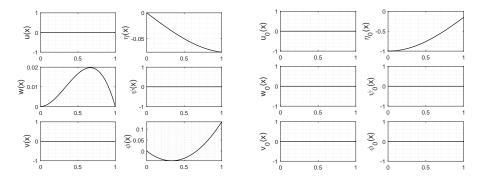
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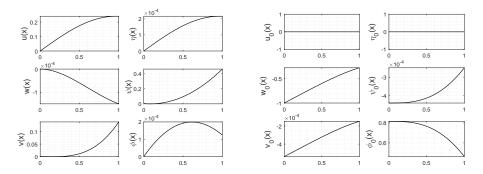
$$\mathcal{H}_{\text{alt}} := \left\{ \Psi \in \mathcal{H} \colon v_0 = w_0 = u_0 = 0, \ u_s = v_s = 0, \ w_1 = w_2 = w_3, \ \eta_1 = \eta_2 = \eta_3 \right\},$$



**Figure:** Plot of the components of the first eigenfuction from  $\mathcal{H}_{alt}$ . Plots are obtained from a finite elements numerical computation and are displayed in the local coordinate system of the corresponding edge. All the results are based on unit materials parameters and beams lengths.

## $\mathcal{H}=\mathcal{H}_{\mathrm{id}}\oplus\mathcal{H}_{\mathrm{alt}}\oplus\mathcal{H}_{\omega}\oplus\mathcal{H}_{\overline{\omega}}$

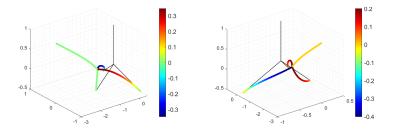
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**Figure:** Plot of (first) eigenfunction fields corresponding to the eigenvalue of multiplicity two in edge's local coordinate system by finite element approximation.

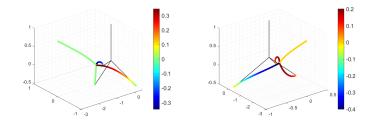
Gregory Berkolaiko & Mahmood Ettehad Elastic Frames: Variational And Differential Formulation

 $\mathcal{H} = \mathcal{H}_{\mathrm{id}} \oplus \mathcal{H}_{\mathrm{alt}} \oplus \mathcal{H}_{\omega} \oplus \mathcal{H}_{\overline{\omega}}$ 



**Figure:** Plot of (left) **first**, and (right) **second** displacement eigenfunction corresponding to eigenvalue of **standard representation** in global coordinate system by finite element approximation. Color bar shows value of in-axis rotation of edges.

## $\mathcal{H} = \mathcal{H}_{\mathrm{id}} \oplus \mathcal{H}_{\mathrm{alt}} \oplus \mathcal{H}_{\omega} \oplus \mathcal{H}_{\overline{\omega}}$



**Questions?** 

# Thanks for your attention !